

CIEM5110-2: FEM, lecture 2.1

Derivation of the finite element method for Timoshenko beam elements

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Objectives

This lecture focuses on the formulation of the finite element method for Timoshenko beams

- Another example of how to go from PDEs to the FE formulation
- Specifically for a case with multiple fields and multiple equations
- Illustration of shear locking
- Stepping stone towards frame analysis for upcoming workshops

The basic ingredients

Equilibrium relations $M_{,x} - V = 0$

$$V_{,x} + q = 0$$

Constitutive relations $M = -EI\kappa$

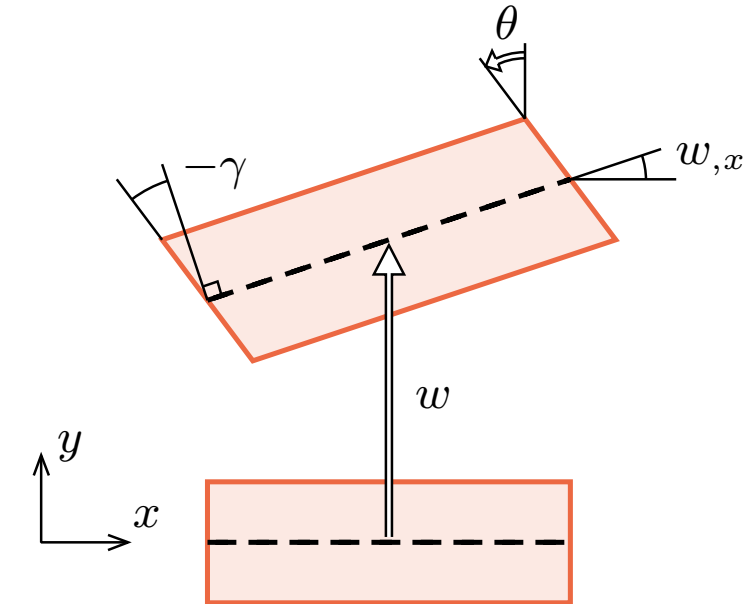
$$V = GA_s\gamma$$

Kinematic relations $\kappa = \theta_{,x}$

$$\gamma = w_{,x} - \theta$$

Strong form equations $-EI\theta_{,xx} - GA_s(w_{,x} - \theta) = 0$

$$GA_s(w_{,xx} - \theta_{,x}) + q = 0$$



Some remarks:

- Notations follow from Track Base except:
 - θ instead of φ for rotations
 - y (and w) are pointing upward
- The strong form has two coupled ODEs
- There are two unknown fields: w and θ

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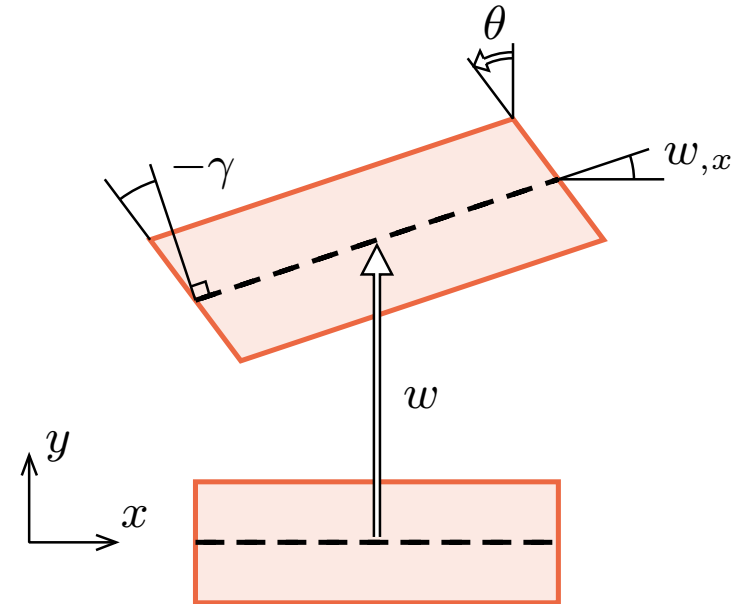
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The alternative would be an Euler beam:

- Single ODE but of 4th order
- Not treated in this unit
- FE derivation is given in the book

Discretized form

The coupled structure remains visible in the system of equations

$$\begin{bmatrix} \mathbf{K}_{\theta\theta} & \mathbf{K}_{\theta w} \\ \mathbf{K}_{w\theta} & \mathbf{K}_{ww} \end{bmatrix} \begin{bmatrix} \mathbf{a}_\theta \\ \mathbf{a}_w \end{bmatrix} = \begin{bmatrix} \mathbf{f}_\theta \\ \mathbf{f}_w \end{bmatrix}$$

with

$$\mathbf{K}_{\theta\theta} = \int_{\Omega} \mathbf{B}_\theta^T E I \mathbf{B}_\theta + \mathbf{N}_\theta^T G A_s \mathbf{N}_\theta \, d\Omega$$

$$\mathbf{K}_{\theta w} = - \int_{\Omega} \mathbf{N}_\theta^T G A_s \mathbf{B}_w \, d\Omega$$

$$\mathbf{K}_{w\theta} = - \int_{\Omega} \mathbf{B}_w^T G A_s \mathbf{N}_\theta \, d\Omega$$

$$\mathbf{K}_{ww} = \int_{\Omega} \mathbf{B}_w^T G A_s \mathbf{B}_w \, d\Omega$$

$$\mathbf{f}_\theta = \int_{\Gamma_M} \mathbf{N}_\theta^T T \, d\Gamma$$

$$\mathbf{f}_w = \int_{\Gamma_V} \mathbf{N}_w^T F \, d\Gamma + \int_{\Omega} \mathbf{N}_w^T q \, d\Omega$$

Extensible beam element

Combining the Timoshenko element with a bar element:

$$\begin{bmatrix} \mathbf{K}_{uu}^e & \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \mathbf{K}_{ww}^e & \mathbf{K}_{w\theta}^e \\ \mathbf{0} & \mathbf{K}_{\theta w}^e & \mathbf{K}_{\theta\theta}^e \end{bmatrix} \begin{bmatrix} \mathbf{a}_u \\ \mathbf{a}_w \\ \mathbf{a}_\theta \end{bmatrix} = \begin{bmatrix} \mathbf{f}_u \\ \mathbf{f}_w \\ \mathbf{f}_\theta \end{bmatrix}$$

with

$$\mathbf{K}_{uu} = \int_{\Omega} \mathbf{B}_u^T E A \mathbf{B}_u d\Omega$$

$$\mathbf{f}_u = \int_{\Gamma_N} \mathbf{N}_u^T F_x d\Gamma + \int_{\Omega} \mathbf{N}_u^T q_x d\Omega$$

$$\mathbf{f}_w = \int_{\Gamma_V} \mathbf{N}_w^T F_y d\Gamma + \int_{\Omega} \mathbf{N}_w^T q_y d\Omega$$

$$\mathbf{f}_\theta = \int_{\Gamma_M} \mathbf{N}_\theta^T T d\Gamma$$

Now there are three coupled PDEs

$$EAu_{,xx} + q_x = 0$$

$$-EI\theta_{,xx} - GA_s(w_{,x} - \theta) = 0$$

$$GA_s(w_{,xx} - \theta_{,x}) + q_y = 0$$

Alternative formulation

Discretized form from the derivation

$$\mathbf{K}^e = \begin{bmatrix} \mathbf{K}_{uu}^e & \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \mathbf{K}_{ww}^e & \mathbf{K}_{w\theta}^e \\ \mathbf{0} & \mathbf{K}_{\theta w}^e & \mathbf{K}_{\theta\theta}^e \end{bmatrix}, \quad \mathbf{a}^e = \begin{bmatrix} \mathbf{a}_u^e \\ \mathbf{a}_w^e \\ \mathbf{a}_\theta^e \end{bmatrix}$$

with

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$$\mathbf{K}_{ww}^e = \int_{\Omega} \mathbf{B}_w^T G A_s \mathbf{B}_w \, d\Omega$$

$$\mathbf{K}_{w\theta}^e = - \int_{\Omega} \mathbf{B}_w^T G A_s \mathbf{N}_\theta \, d\Omega$$

$$\mathbf{K}_{\theta w}^e = - \int_{\Omega} \mathbf{N}_\theta^T G A_s \mathbf{B}_w \, d\Omega$$

$$\mathbf{K}_{\theta\theta}^e = \int_{\Omega} \mathbf{B}_\theta^T E I \mathbf{B}_\theta + \mathbf{N}_\theta^T G A_s \mathbf{N}_\theta \, d\Omega$$

Alternative: collect all deformations in a single vector

$$\boldsymbol{\varepsilon} \equiv \begin{Bmatrix} \varepsilon \\ \gamma \\ \kappa \end{Bmatrix} = \begin{Bmatrix} u_{,x} \\ w_{,x} - \theta \\ \theta_{,x} \end{Bmatrix} = \mathbf{B} \mathbf{a}^e$$

with

$$\mathbf{B} = \begin{bmatrix} \mathbf{B}_u & \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \mathbf{B}_w & -\mathbf{N}_\theta \\ \mathbf{0} & \mathbf{0} & \mathbf{B}_\theta \end{bmatrix}$$

The stiffness matrix takes a familiar form

$$\mathbf{K}^e = \int_{\Omega} \mathbf{B}^T \mathbf{D} \mathbf{B} \, d\Omega$$

with

$$\mathbf{D} = \begin{bmatrix} EA & 0 & 0 \\ 0 & GA & 0 \\ 0 & 0 & EI \end{bmatrix}$$

Shear locking (2-node Timoshenko beam element)

Objective: pure bending deformation ($\kappa \neq 0, \varepsilon = 0, \gamma = 0$):

$$\text{Node 1: } u_1 = w_1 = 0, \quad \theta_1 = -\frac{L^e}{2}\kappa, \quad N_1 = 1 - \frac{x}{L^e}$$

$$\text{Node 2: } u_2 = w_2 = 0, \quad \theta_2 = \frac{L^e}{2}\kappa, \quad N_2 = \frac{x}{L^e}$$

$$\text{Interpolations: } u(x) = 0, \quad w(x) = 0, \quad \theta(x) = N_i\theta_i = \kappa x - \frac{L^e}{2}\kappa$$

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$$\text{Strains: } \kappa = \theta_{,x} = \kappa \checkmark, \quad \varepsilon = u_{,x} = 0 \checkmark, \quad \gamma = w_{,x} - \theta = -\kappa x + \frac{L^e}{2}\kappa \times$$

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Solution: reduced integration

- Only evaluate shear strain at the center (where $\gamma = 0$ above)
- In Timoshenko beam: for terms that are not related to γ , 1 point is enough for exact integration
- Using only 1 point removes shear locking without side effects

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Alternative solution: mixed interpolation (quadratic for w , linear for θ)

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